Lecture 4 Honeycombs Notes, 3.054

Honeycombs-In-plane behavior

- Prismatic cells
- Polymer, metal, ceramic honeycombs widely available
- Used for sandwich structure cores, energy absorption, carriers for catalysts
- Some natural materials (e.g. wood, cork) can be idealized as honeycombs
- Mechanisms of deformation and failure in hexagonal honeycombs parallel those in foams
 simpler geometry unit cell easier to analyze
- Mechanisms of deformation in triangular honeycombs parallel those in 3D trusses (lattice materials)

Stress-strain curves and Deformation behavior: In-Plane

Compression

- 3 regimes linear elastic bending
 - stress plateau buckling
 - yielding
 - brittle crushing
 - densification cell walls touch
- Increasing t/l E^* σ^* ϵ_D

Honeycomb Geometry



Gibson, L. J., and M. F. Ashby. *Cellular Solids: Structure and Properties*. 2nd ed. Cambridge University Press, 1997. Figure courtesy of Lorna Gibson and Cambridge University Press.



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Deformation mechanisms

Bending X₁ Loading

Bending Shear



Gibson, L. J., and M. F. Ashby. *Cellular Solids: Structure and Properties*. 2nd ed. Cambridge University Press, © 1997. Figure courtesy of Lorna Gibson and Cambridge University Press.



Plastic collapse in an aluminum honeycomb

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Stress-Strain Curve



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Tension

- Linear elastic bending
- Stress plateau exists only if cell walls yield
 - no buckling in tension
 - brittle honeycombs fracture in tension

Variables affecting honeycomb properties

• Relative density
$$\frac{\rho^*}{\rho_s} = \frac{\left(\frac{t}{l}\right)\left(\frac{h}{l}+2\right)}{2\cos\theta\left(\frac{h}{l}\sin\theta\right)} = \frac{2}{-\frac{3}{3}}\frac{t}{l}$$
 regular hexagons

- Solid cell wall properties: $\rho_s, E_s, \sigma_{ys}, \sigma_{fs}$
- Cell geometry: $h/l, \theta$



In-plane properties

Assumptions:

- t/l small ((ρ_c^*/ρ_s) small) neglect axial and shear contribution to deformation
- Deformations small neglect changes in geometry
- Cell wall linear elastic, isotropic

Symmetry

 \bullet Honeycombs are orthotropic — rotate 180° about each of three mutually perpendicular axes and structure is the same

Linear elastic deformation

$$\begin{bmatrix} \epsilon_1\\ \epsilon_2\\ \epsilon_3\\ \epsilon_4\\ \epsilon_5\\ \epsilon_6 \end{bmatrix} = \begin{bmatrix} 1/E_1 & -\nu_{21}/E_2 & -\nu_{31}/E_2 & 0 & 0 & 0\\ -\nu_{12}/E_1 & 1/E_2 & -\nu_{32}/E_3 & 0 & 0 & 0\\ -\nu_{13}/E_1 & -\nu_{23}/E_2 & -1/E_3 & 0 & 0 & 0\\ 0 & 0 & 0 & 1/G_{23} & 0 & 0\\ 0 & 0 & 0 & 0 & 1/G_{13} & 0\\ 0 & 0 & 0 & 0 & 0 & 1/G_{12} \end{bmatrix} \begin{bmatrix} \sigma_1\\ \sigma_2\\ \sigma_3\\ \sigma_4\\ \sigma_5\\ \sigma_6 \end{bmatrix}$$

• Matrix notation: $\begin{aligned} \epsilon_1 &= \epsilon_{11} \quad \epsilon_4 = \gamma_{23} \quad \sigma_1 = \sigma_{11} \quad \sigma_4 = \sigma_{23} \\ \epsilon_2 &= \epsilon_{22} \quad \epsilon_5 = \gamma_{13} \quad \sigma_2 = \sigma_{22} \quad \sigma_5 = \sigma_{13} \\ \epsilon_3 &= \epsilon_{33} \quad \epsilon_6 = \gamma_{12} \quad \sigma_3 = \sigma_{33} \quad \sigma_6 = \sigma_{12} \end{aligned}$

In-plane
$$(x_1 - x_2)$$
: 4 independent elastic constants:
 $E_1 \quad E_2 \quad \nu_{12} \quad G_{12}$
and compliance matrix symmetric $\frac{-\nu_{12}}{E_1} = \frac{-\nu_{21}}{E_2}$ (reciprocal relation)

$$\begin{bmatrix} \text{notation for Poisson s ratio:} & \nu_{ij} = \frac{-\epsilon_j}{\epsilon_i} \end{bmatrix}$$

Young's modulus in x_1 direction

•



In-Plane Deformation: Linear Elasticity

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M diagram: 2 cantilevers of length $\mathrm{l}/\mathrm{2}$

$$\delta = 2 \cdot \frac{P \sin \theta (l/2)^3}{3E_s I}$$
$$= \frac{2 P l^3 \sin \theta}{24 E_s I}$$
$$\delta = \frac{P l^3 \sin \theta}{12 E_s I} \qquad I = \frac{b t^3}{12}$$

Combining:

$$E_{1}^{*} = \frac{\sigma_{1}}{\epsilon_{1}} = \frac{P}{(h+l\,\sin\theta)\,b} \frac{l\,\cos\theta}{\delta\sin\theta}$$
$$= \frac{P}{(h+l\,\sin\theta)\,b} \frac{l\,\cos\theta}{P\,l^{3}\,\sin^{2}\theta} \,12 \,E_{s} \frac{b\,t^{3}}{12}$$
$$E_{1}^{*} = E_{s} \left(\frac{t}{l}\right)^{3} \frac{\cos\theta}{(h/l+\sin\theta)\,\sin^{2}\theta} = \frac{4}{3} \left(\frac{t}{l}\right)^{3} E_{s} \qquad \begin{array}{c} \text{regular} \\ \text{hexagons} \\ h/l=1 \,\theta = 30^{\circ} \end{array}$$

solid relative cell geometry property density

Poisson's ratio for loading in x_1 direction



$$\epsilon_1 = \frac{\delta \sin \theta}{l \cos \theta} \qquad \epsilon_2 = \frac{\delta \cos \theta}{h + l \sin \theta} \quad \text{(lengthens)}$$
$$\nu_{12}^* = \frac{\delta \cos \theta}{h + l \sin \theta} \left(\frac{l \cos \theta}{\delta \sin \theta}\right) = \frac{\cos^2 \theta}{(h/l + \sin \theta) \sin \theta}$$

- ν_{12}^* depends <u>ONLY</u> on cell geometry (h/l, θ), not on E_s , t/l
- Regular hexagonal cells: $\nu_{12}^* = 1$
- ν can be negative for $\theta < 0$

e.g. h/l=2
$$\theta = -30^{\circ}$$
 $\nu_{12}^* = \frac{3/4}{(3/2)(-1/2)} = -1$

 $\mathbf{E_2^*} \quad \nu_{12}^* \quad \mathbf{G_{12}^*}$

• Can be found in similar way; results in book

Compressive strength (plateau stress)

• Cell collapse by:



• buckling of vertical struts throughout honeycomb



- localization of yield
- as deformation progresses, propagation of failure band



• peaks and valleys correspond to fracture of individual cell walls

Plateau stress: elastic buckling, σ_{el}^*

- Elastomeric honeycombs cell collapse by elastic buckling of walls of length h when loaded in x_2 direction
- No buckling for σ_1 ; bending of inclined walls goes to densification



Euler buckling load

$$P_{cr} = \frac{n^2 \, \pi^2 \, E_s I}{h^2}$$



Elastic Buckling

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- Here, constraint n depends on stiffness of adjacent inclined members
- Can find elastic line analysis (see appendix if interested)
- Rotational stiffness at ends of column, h, matched to rotational stiffness of inclined members

• Find
$$n/l=1$$
 1.5 2
 $n=0.686$ 0.760 0.860
and $(\sigma_{el}^*)_2 = \frac{P_{cr}}{2l\cos\theta b} = \frac{n^2 \pi^2 E_s}{h^2 2l\cos\theta b} \frac{bt^3}{12}$
 $(\sigma_{el}^*)_2 = \frac{n^2 \pi^2}{24} E_s \frac{(t/l)^3}{(h/l)^2\cos\theta}$

regular hexagons: and since strain at buckling $(\sigma_{el}^*)_2 = 0.22 E_s(t/l)^3$ $E_2^* = 4/\overline{3} E_s(t/l)^3 = 2.31 E_s(t/l)^3$ $(\epsilon_{el}^*)_2 = 0.10$, for regular hexagons, independent of E_s , t/l

Plateau stress: plastic yielding, σ_{pl}^{*}

- Failure by yielding in cell walls
- Yield strength of cell walls = σ_{ys}
- Plastic hinge forms when cross-section fully yields
- Once stress outer fiber= σ_{ys} , yielding begins and then progresses through the section, as the load increases



- When section fully yielded (right figure), form plastic hinge
- Section rotates like a pin

Plastic Collapse

Figure removed due to copyright restrictions. See Figure 8: L. J. Gibson, M. F. Ashby, et al. "The Mechanics of Two-Dimensional Cellular Materials." • Moment at formation of plastic hinge (plastic moment, M_p):

$$M_p = \left(\sigma_{ys} \, \frac{b \, t}{2}\right) \left(\frac{t}{2}\right) = \frac{\sigma_{ys} \, b \, t^2}{4}$$

• Applied moment, from applied stress

$$2M_{app} - PL\sin\theta = 0$$

$$M_{app} = \frac{P l\sin\theta}{2}$$

$$\sigma_1 = \frac{P}{(h+l\sin\theta)b}$$

$$P \rightarrow M_{app} = \sigma_1 (h+l\sin\theta) b \frac{l\sin\theta}{2}$$

• Plastic collapse of honeycomb at $(\sigma_{pl}^*)_1$, when $M_{app} = M_p$

$$(\sigma_{pl}^*)_1 (h+l\sin\theta) \not b \frac{l\sin\theta}{\not 2} = \sigma_{ys} \frac{\not b t^2}{\not 4 2}$$
$$\left[(\sigma_{pl}^*)_1 = \sigma_{ys} \left(\frac{t}{l}\right)^2 \frac{1}{2(h/l+\sin\theta)\sin\theta} \right]$$

regular hexagons:
$$(\sigma_{pl}^*)_1 = \frac{2}{3} \sigma_{ys} \left(\frac{t}{l}\right)^2$$

similarly, $(\sigma_{pl}^*)_2 = \sigma_{ys} \left(\frac{t}{l}\right)^2 \frac{1}{2\cos^2\theta}$

- For thin-walled honeycombs, elastic buckling can precede plastic collapse (for σ_2)
- Elastic buckling stress = plastic collapse stress $(\sigma_{el}^*)_2 = (\sigma_{pl}^*)_2$

$$\frac{n^2 \pi^2}{24} E_s \frac{(t/l)^3}{(h/l)^2 \cos \theta} = \frac{\sigma_{ys}(t/l)^2}{2 \cos^2 \theta}$$

$$(t/l)_{\text{critical}} = \frac{12 (h/l)^2}{n^2 \pi^2 \cos \theta} \left(\frac{\sigma_{ys}}{E_s}\right)$$

regular hexagons: $(t/l)_{\text{critical}} = 3 \frac{\sigma_{ys}}{E_s}$

- E.g. metals $\sigma_{ys}/E_s \sim .002$ $(t/l)_{\rm critical} \sim 0.6\%$
 - $\circ\,$ most metal honeycomb denser than this polymer $\sigma_{ys}/E_s\sim 3-5\%~(t/l)_{\rm critical}\sim 10\text{-}15\%$
 - $\circ\,$ low density polymers with yield point may buckle before yield

Plastic stress: brittle crushing, $(\sigma_{cr}^*)_1$

- Ceramic honeycombs fail in brittle manner
- Cell wall bending stress reaches modulus of rapture wall fracture loading in x_1 direction:

 $P = \sigma_1 \left(h + l \sin \theta \right) b$ $\sigma_{fs} =$ modulus of rupture of cell wall

$$M_{\text{max. applied}} = \frac{P \, l \, \sin \theta}{2} = \frac{\sigma_1 \left(h + l \sin \theta\right) b \, l \sin \theta}{2}$$

Moment at fracture, M_f



$$M_f = \left(\frac{1}{2} \sigma_{fs} b \frac{t}{2}\right) \left(\frac{2}{3} t\right) = \frac{\sigma_{fs} b t^2}{6}$$
$$(\sigma_{cr}^*)_1 = \sigma_{fs} \left(\frac{t}{l}\right)^2 \frac{1}{3 (h/l + \sin \theta) \sin \theta}$$

regular hexagons: $(\sigma_{cr}^*)_1 = \frac{4}{9} \sigma_{fs} \left(\frac{t}{l}\right)^2$

Brittle Crushing



Gibson, L. J., and M. F. Ashby. *Cellular Solids: Structure and Properties*. 2nd ed. Cambridge University Press, © 1997. Figure courtesy of Lorna Gibson and Cambridge University Press.

Tension

- No elastic buckling
- Plastic plateau stress approx. same in tension and compression (small geometric difference due to deformation)
- Brittle honeycombs: fast fracture

Fracture toughness

Assume:

- crack length large relative to cell size (continuum assumption)
 - axial forces can be neglected
 - cell wall material has constant modulus of rapture, σ_{fs}

Continuum: crack of length 2c in a linear elastic solid material normal to a remote tension stress σ_1 creates a local stress field at the crack tip



$$\sigma_{\text{local}} = \sigma_l = \frac{\sigma_1 \quad \overline{\pi \ c}}{\overline{2\pi \ r}}$$

Fracture Toughness



Gibson, L. J., and M. F. Ashby. *Cellular Solids: Structure and Properties*. 2nd ed. Cambridge University Press, © 1997. Figure courtesy of Lorna Gibson and Cambridge University Press.

Honeycomb: cell walls bent fail when applied moment = fracture moment

$$M_{app} \quad P \ l \quad \text{on wall A}$$

$$M_{app} \quad P \ l \quad \sigma_l \ l^2 \ b \quad \frac{\sigma_1 \quad \overline{c} \ l^2 \ b}{\overline{l}} \quad \sigma_{fs} \ b \ t^2$$

$$(\sigma_f^*)_1 \quad \sigma_{fs} \left(\frac{t}{\overline{l}}\right)^2 \sqrt{\frac{l}{\overline{c}}}$$

$$K_{IC}^* = \sigma_f^* \quad \overline{\pi c} = c \ \sigma_{fs} \left(\frac{t}{\overline{l}}\right)^2 \quad \overline{l}$$
depends on cell size, l!



Summary: hexagonal honeycombs, in-plane properties

- Linear elastic moduli: E_1^* E_2^* ν_{12}^* G_{12}^*
- $(\sigma^*_{el})_2 \ \sigma^*_{pl} \ \sigma^*_{cr}$ elastic buckling • Plateau stresses (compression) plastic yield
 - brittle crushing
- K_{IC}^* • Fracture toughness (tension)
 - brittle fracture

Honeycombs: In-plane behavior — triangular cells



• Triangulated structures - trusses

- Can analyze as pin-jointed (no moment at joints)
- Forces in members all axial (no bending)
- If joints fixed and include bending, difference $\sim 2\%$
- Force in each member proportional to P



$$\begin{split} \sigma \propto \frac{P}{l \, b} & \epsilon \propto \frac{\delta}{l} & \delta \propto \frac{P \, l}{A \, E_s} \text{-axial shortening: Hooke's law} \\ E^* \propto \frac{\sigma}{\epsilon} \propto \frac{P}{l \, b} \frac{l}{\delta} & \propto \frac{P}{b} \frac{b \, t \, E_s}{P \, l} \propto E_s \left(\frac{t}{l}\right) \\ E^* &= c \, E_s \, (t/l) \end{split}$$

exact calculation: $E^* = 1.15 E_s (t/l)$ for equilateral triangles

Square and Triangular Honeycombs



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